

**FOUNDATION NOTES**

- F1 NOT USED
- F2 BEAR ALL FOOTINGS ON SOIL HAVING A MINIMUM NET ALLOWABLE BEARING CAPACITY OF 2.5 KIPS PER SQUARE FOOT
- F3 THE FOUNDATIONS HAVE BEEN DESIGNED TO THE REQUIREMENTS SET FORTH IN THE GEOTECHNICAL REPORT.
- F4 THE SUBSURFACE CONDITIONS DESCRIBED ON THE DRAWINGS, TEST BORINGS, OR TEST PITS REPRESENT CONDITIONS ONLY AT THOSE SPECIFIC LOCATIONS AT THE PARTICULAR TIME THEY WERE MADE.
- F5 REMOVE ALL ORGANIC AND UNSUITABLE MATERIAL PRIOR TO PLACING FILL. PLACE FILL IN HORIZONTAL LAYERS NOT TO EXCEED 8 INCHES IN LOOSE THICKNESS AND COMPACT TO A MINIMUM OF 95% MOISTURE PROCTOR DENSITY AT OPTIMUM MOISTURE CONTENT. IF ACCEPTABLE TO THE OWNER'S GEOTECHNICAL ENGINEER, ON-SITE MATERIALS THAT MEET PROJECT SPECIFICATION REQUIREMENTS MAY BE USED FOR ENGINEERED FILL IF MAINTAINED AT OPTIMUM MOISTURE CONTENT AND COMPACTED TO THE ABOVE CRITERIA. SELECT BORROW MATERIALS WILL BE REQUIRED WHEN ON-SITE MATERIALS ARE UNSUITABLE OR CANNOT BE COMPACTED TO THE CRITERIA STATED ABOVE.
- F6 EXCAVATION TO SUITABLE BEARING SUBGRADES MAY PROCEED BY CONVENTIONAL METHODS TO WITHIN 2.5 FEET OF THE PROPOSED FINAL SUBGRADES. PERFORM EXCAVATION TO FINAL SUBGRADE USING A BACKHOE EQUIPPED WITH A SMOOTH BLADE TO MINIMIZE DISTURBANCE OF THE BEARING SUBGRADE. FINISH ALL FOOTING EXCAVATIONS BY HAND.
- F7 DO NOT EXTEND THE GENERAL EXCAVATION ACROSS THE SITE DEEPER THAN 1'-0" BELOW THE SLAB-ON-GRADE SUBGRADE ELEVATION. PERFORM THE EXCAVATIONS FOR SPREAD FOOTINGS, WALLS, PITS, ETC. ON AN INDIVIDUAL, LOCALIZED BASIS DOWN FROM THE SLAB-ON-GRADE SUBGRADE ELEVATION.
- F8 PROVIDE POSITIVE PROTECTION FOR ALL EXCAVATION SLOPES AGAINST INSTABILITY AND DEGRADATION DUE TO RAIN, WIND, SNOW OR ICE.
- F9 RETAIN THE PERIMETER OF THE GENERAL EXCAVATION WITH A SOIL RETENTION SYSTEM AS NECESSARY. THE DESIGN, INSTALLATION, MAINTENANCE AND REMOVAL OF THE SYSTEM IS THE RESPONSIBILITY OF THE CONTRACTOR. PROVIDE ALL MEASURES AND PRECAUTIONS NECESSARY TO MINIMIZE SETTLEMENT OF EXISTING OR NEW CONSTRUCTION INSIDE OR OUTSIDE OF THE PROJECT LIMITS. REPAIR ANY DAMAGE TO NEW OR EXISTING CONSTRUCTION INSIDE OR OUTSIDE PROJECT LIMITS CAUSED BY CONSTRUCTION TECHNIQUES OR MOVEMENTS OF THE SOIL RETENTION SYSTEM.
- F10 ALLOW THE OWNER'S GEOTECHNICAL ENGINEER TO INSPECT ALL FINISHED EXCAVATIONS AND BEARING SUBGRADES BEFORE PLACING CONCRETE.
- F11 DO NOT PLACE CONCRETE IN ANY EXCAVATION CONTAINING FRESH WATER, FROST, ICE OR FROZEN GROUND. PROVIDE ALL MEASURES NECESSARY TO PREVENT FROST OR ICE FROM PENETRATING ANY FOOTING OR SLAB SUBGRADE, BOTH BEFORE AND AFTER CONCRETE PLACEMENT AND UNTIL SUCH SUBGRADES ARE FULLY PROTECTED BY THE PERMANENT BUILDING STRUCTURE.
- F12 USE SIDE FORMS FOR ALL FOOTINGS AND GRADE BEAMS. CLEAN REINFORCEMENT IMMEDIATELY PRIOR TO PLACING CONCRETE. PLACE THE CONCRETE FOR EACH FOOTING IN ONE CONTINUOUS POUR. LIMIT BASEMENT WALL FOUR LENGTHS TO 45 FEET.
- F13 SAWCUT SLABS IN THE PATTERN SHOWN ON PLAN START SAWCUTTING AS SOON AS THE SAW WILL NOT RATTLE, EXCESS OR DISCLOSE AGGREGATE, BUT IN NO CASE MORE THAN 12 HOURS AFTER THE SLAB IS PLACED.
- F14 BACKFILL AGAINST FOUNDATION WALLS BELOW GRADE SO THAT THE DIFFERENCE IN THE FILL LEVEL ON OPPOSITE SIDES OF THE WALL DOES NOT EXCEED 1'-0" AT ANY TIME. BRACE ALL FOUNDATION WALLS DURING THE OPERATION OF BACKFILLING AND COMPACTING. LEAVE BRACING IN POSITION UNTIL PERMANENT RESTRAINTS BECOME EFFECTIVE.
- F15 THE EXPOSED SUBGRADE SOILS MAY BE SENSITIVE TO DISTURBANCE AND STRENGTH DEGRADATION WHEN HIGH MOISTURE CONTENTS ARE PRESENT. MINIMIZE CONSTRUCTION TRAFFIC OVER EXPOSED SUBGRADES. DO NOT POUD WATER ON THE SUBGRADES. CONTROL SURFACE AND GROUND WATER BY PROPER SITE GRADING, PERMEABLE OUTLET TRENCHES, AND SUMP AND PUMP METHODS OF DRAINAGE. CONSTRUCT ALL OUTLET TRENCHES AND SUMPS OUTSIDE THE INFLUENCE OF PROPOSED FOUNDATIONS.

**STRUCTURAL CONCRETE NOTES**

- C1 NOT USED
- C2 A QUALITY CONTROL PROGRAM OF FIELD TESTING AND INSPECTION WILL BE PERFORMED ON ALL STRUCTURAL CONCRETE WORK IN ACCORDANCE WITH THE SPECIFICATIONS. SCHEDULE WORK AND PROVIDE ACCESS TO ALLOW THE TESTING REQUIREMENTS TO BE COMPLETED.
- C3 SUBMIT ENGINEERED CONCRETE MIX DESIGNS, INCLUDING REQUIRED BACKUP DATA, FOR EACH TYPE OF CONCRETE PROPOSED FOR USE TO THE ARCHITECT/ENGINEER FOR REVIEW. ALLOW ADEQUATE TIME FOR REVIEW PRIOR TO PERFORMING CONCRETE WORK.
- C4 DETAIL, FABRICATE, LABEL, SUPPORT AND PLACE ALL CONCRETE REINFORCEMENT IN ACCORDANCE WITH AC 318 "MANUAL OF ENGINEERING AND PLACING DRAWINGS FOR REINFORCED CONCRETE STRUCTURES" AND AC 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE", LATEST EDITIONS.
- C5 SUBMIT DETAILED SHOP DRAWINGS INDICATING REINFORCEMENT SIZE, SPACING AND PLACEMENT TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION. INCLUDE DETAILS AND LOCATIONS OF ALL CURBS, CONSTRUCTION JOINTS, SLAB DEPRESSIONS, SLEEVES, OPENINGS, ETC.
- C6 REFER TO THE FOUNDATION NOTES FOR REQUIREMENTS PARTICULAR TO FOUNDATION AND SLAB-ON-GRADE WORK.
- C7 CLEAN AND MOISTEN ALL CONSTRUCTION JOINTS IMMEDIATELY PRIOR TO PLACING FRESH CONCRETE.
- C8 LAP WELDED WIRE FABRIC ONE FULL MESH PANEL (TYPICALLY) OR TWO FULL MESH PANELS (AT CONSTRUCTION JOINTS) AND THE SECURELY.
- C9 PROVIDE DOWELS, WHERE REQUIRED, TO MATCH MAIN REINFORCING SIZE AND SPACING.
- C10 DO NOT USE CALCIUM CHLORIDE IN ANY CONCRETE.
- C11 HORIZONTAL JOINTS IN CONCRETE POURS ARE NOT PERMITTED.
- C12 ALLOW THE OWNER'S TESTING AGENCY TO REVIEW THE REINFORCING PLACEMENT PRIOR TO PLACING ANY CONCRETE. PROVIDE ADEQUATE ADVANCE NOTICE.
- C13 REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR CURBS, PADS, DEPRESSIONS, WALL-SLAB OPENINGS, SPECIAL FLOOR FINISHES, ETC.
- C14 NOT USED
- C15 PROVIDE AIR-ENTRAINING IN CONCRETE AS SET FORTH IN THE SPECIFICATIONS.
- C16 PROVIDE ONLY THOSE OPENINGS INDICATED ON THE REVENED SHOP DRAWINGS.
- C17 REFER TO ACI 318, CH. 7.7 FOR MINIMUM CONCRETE COVER REQUIREMENTS.
- C18 REFER TO ACI 308 FOR REQUIREMENTS FOR PLACING CONCRETE IN HOT WEATHER AND TO AC 308 FOR REQUIREMENTS FOR PLACING CONCRETE IN COLD WEATHER.
- C19 ON STEEL FRAMED FLOORS, PROVIDE ADDITIONAL CONCRETE AS NECESSARY TO FINISH THE FLOORS TO WITHIN SPECIFIED TOLERANCES WHILE ACCOUNTING FOR METAL DECK AND STEEL BEAM DEFLECTIONS. ALLOW FOR AN AVERAGE OF AT LEAST 1/2 INCH OF CONCRETE FOR EACH FLOOR.
- C20 PROVIDE ONLY CONCRETE AND REINFORCING MATERIALS OF THE TYPES AND GRADES LISTED IN THE TABLE BELOW.

CONCRETE	F'C (PSI)	UNIT WEIGHT (PCF)
FOOTINGS, FOUNDATIONS	4000	145
SLAB ON GRADE	3000	145
SLAB ON METAL DECK	3000	115

REINFORCING	GRADE
TYPICAL BARS	ASTM A-615, GRADE 60
WELDED WIRE FABRIC	ASTM A-185
STEEL FIBERS	ASTM A-820 TYPE I

**STRUCTURAL STEEL NOTES**

- S1 NOT USED
- S2 A QUALITY CONTROL PROGRAM OF SHOP AND FIELD TESTING AND INSPECTION WILL BE PERFORMED ON STRUCTURAL STEEL FABRICATION, ERECTION, AND CONNECTIONS IN ACCORDANCE WITH THE SPECIFICATIONS. SCHEDULE WORK AND PROVIDE ACCESS TO ALLOW THE TESTING REQUIREMENTS TO BE COMPLETED.
- S3 DETAIL, FABRICATE AND ERECT STRUCTURAL STEEL IN CONFORMANCE WITH THE AISC SPECIFICATIONS AND CODES, LATEST EDITIONS.
- S4 PERFORM ALL WELDING USING CERTIFIED WELDERS AND IN ACCORDANCE WITH THE AISC STRUCTURAL WELDING CODE - STEEL, LATEST EDITION. COMPLY WITH AISC SPECIFICATION SECTION 4.2 FOR MINIMUM FILL METAL SIZE, BUT DO NOT USE LESS THAN A 1/4 INCH FILLET UNLESS SPECIFICALLY NOTED ON THE DRAWINGS.
- S5 SUBMIT ENGINEERED AND CHECKED SHOP DRAWINGS TO THE ARCHITECT/ENGINEER FOR REVIEW. SHOW SHOP FABRICATION DETAILS, FIELD ASSEMBLY DETAILS, AND ERECTION DIAGRAMS FOR ALL STRUCTURAL STEEL. SCHEDULE SUBMISSIONS TO ALLOW ADEQUATE TIME FOR REVIEW PRIOR TO FABRICATION.
- S6 DESIGN AND DETAILING OF THE CONNECTIONS IS THE RESPONSIBILITY OF THE FABRICATOR. USE RATIONAL ENGINEERING DESIGN AND STANDARD PRACTICE FOR THE CRITERIA SET FORTH IN THE CONTRACT DOCUMENTS. THE DETAILS SHOWN ON THE DRAWINGS ARE CONCEPTUAL AND DO NOT INDICATE THE REQUIRED WELD SIZES OR NUMBER OF BOLTS UNLESS SPECIFICALLY NOTED. SUBMIT CALCULATIONS FOR REVIEW STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF NEW YORK.
- S7 BEAM-TO-COLUMN CONNECTIONS ARE MOMENT CONNECTIONS UNLESS INDICATED ON THE DRAWINGS. PROVIDE MOMENT CONNECTIONS WHICH DEVELOP THE FULL MOMENT CAPACITY OF THE BEAM UNLESS OTHER CRITERIA ARE NOTED OR SCHEDULED ON THE DRAWINGS.
- S8 DESIGN ALL OTHER CONNECTIONS AS SIMPLE SHEAR CONNECTIONS UTILIZING HIGH STRENGTH BOLTS IN SHEAR/BEARING TYPE CONNECTIONS TO PROPERLY ACCOUNT FOR THE INHERENT RIGIDITY OF THIS CONNECTION TYPE.
- S9 DESIGN SINGLE SHEAR PLATE CONNECTIONS, IF SELECTED, USING A-36 PLATE (MIN. 3/8" THICKNESS) WITH LONG OR SHORT SLOTTED HOLES FOR THE BEAM REACTIONS LISTED. USE RATIONAL ENGINEERING DESIGN TO PROPERLY ACCOUNT FOR THE INHERENT RIGIDITY OF THIS CONNECTION TYPE.
- S10 DO NOT USE BOLTS OF THE SAME DIAMETER WITH DIFFERING STEEL GRADES (A325, A490, ETC.). BOLT DIAMETERS MUST VARY BY AT LEAST 1/4 INCH.
- S11 BEAM REACTIONS ARE NOTED ON THE DRAWINGS AT THE ENDS OF THE BEAM.
- S12 PROVIDE A MINIMUM OF TWO (2) BOLTS AT EACH FAYING SURFACE.
- S13 DETAIL ALL BEAMS FRAMING INTO CONCRETE WALLS TO ALLOW FOR HORIZONTAL FILL TOLERANCES AND THERMAL MOVEMENT. PROVIDE CONNECTION DETAILS REQUIRED BY THE SPECIFIC CONSTRUCTION SEQUENCES.
- S14 PROVIDE SUITABLE BEARING PLATES AND ANCHOR BOLTS FOR BEAMS, OR GROSS BEAM BEAR ON WALLS. LOCATE ITEMS USING TEMPLATES OR SIMILAR METHODS. SET ALL PLATES IN FULL BEDS OF NON-SHRINK GROUT. COMPLETELY FILL ALL BEAM AND COLUMN POCKETS WITH CONCRETE PRIOR TO CASTING CONCRETE ABOVE.
- S15 FABRICATE ALL BEAMS, WITH THE NATURAL CAMBER UP. PROVIDE ANY ADDITIONAL CAMBER SHOWN ON THE STRUCTURAL DRAWINGS.
- S16 AFTER FABRICATION, CLEAN STEEL OF ALL RUST, LOOSE MILL SCALE, DIRT, OIL, GREASE OR OTHER FOREIGN MATERIALS.
- S17 REFER TO THE ARCHITECTURAL DRAWINGS FOR THE REQUIRED FIRE RATINGS AND UL ASSEMBLY NUMBERS. APPLY UL APPROVED FIREPROOFING TO THE STRUCTURAL STEEL MEMBERS, AS NECESSARY TO ACHIEVE THE REQUIRED FIRE RATINGS. REMOVE, IN THE FIELD, ALL LOOSE MILL SCALE, RUST, OIL, GREASE OR OTHER BOND-INHERING FOREIGN SUBSTANCE PRIOR TO THE APPLICATION OF FIREPROOFING.
- S18 DO NOT FIELD CUT ANY STRUCTURAL STEEL WITHOUT THE PRIOR REVIEW AND ACCEPTANCE OF THE ARCHITECT/ENGINEER. CLEARLY INDICATE ON THE SHOP DRAWINGS SUBMITTED FOR REVIEW ANY MEMBER OPENINGS REQUIRED BY OTHER TRADES.
- S19 ERECTION PROCEDURES, SEQUENCES AND COORDINATION OF WORK WITH OTHER TRADES IS THE RESPONSIBILITY OF THE CONTRACTOR. PROVIDE ANY ADDITIONAL STEEL REQUIRED FOR ERECTION PURPOSES AT NO COST TO THE OWNER. REMOVE THE ADDITIONAL STEEL UNLESS DIRECTED OTHERWISE BY THE OWNER IN WRITING.
- S20 PROVIDE TEMPORARY BRACING AND SHORING AS REQUIRED FOR THE SAFETY, STABILITY AND ALIGNMENT OF THE STRUCTURE. LEAVE TEMPORARY BRACING IN PLACE FOR AS LONG AS NECESSARY. PERFORM FINAL BOLTING AND WELDING ONLY ON THOSE PORTIONS OF THE STRUCTURE THAT HAVE BEEN ALIGNED AND PLUMBED WITHIN THE SPECIFIED TOLERANCES.
- S21 PROVIDE NEW MATERIAL CONFORMING TO THE FOLLOWING REQUIREMENTS FOR ALL STRUCTURAL STEEL:

- C21 PROVIDE BLENDED REINFORCING FIBERS ON ALL CONCRETE SLABS ON METAL DECK. BLEND TO BE MIX OF 24.0 LB/CY ASTM A820 TYPE I STEEL FIBERS AND 1.50 LB/SY GRADED POLYPROPYLENE FIBERS OF VARIOUS LENGTHS AND THICKNESS. A REPRESENTATIVE FROM THE FIBER MANUFACTURER WILL NEED TO PRESENT AT THE INITIAL CONCRETE PLACEMENT TO ADVISE ON ANY SPECIAL PLACING & FINISHING TECHNIQUES. COMPLY WITH ASTM C-1116 TYPE II 4.1.3

**STRUCTURAL METAL DECK NOTES**

- M1 NOT USED
- M2 FABRICATE METAL DECKING FROM STEEL TYPE ASTM A563, STRUCTURAL QUALITY HAVING A MINIMUM YIELD STRENGTH OF 33,000 PSI. HOT DIP GALVANIZE ROOF DECK TO G90 REQUIREMENTS AND ALL OTHER METAL DECK TO G90 REQUIREMENTS. FORM ROOF METAL DECK WITH TELESCOPED ENDS TO LAP ENDS OF SHEETS A MINIMUM OF 2 INCHES.
- M3 SUBMIT ENGINEERED AND CHECKED SHOP DRAWINGS INDICATING LOCATION, GAGE AND SIZE OF EACH PIECE OF DECKING. CLEARLY SHOW WELD DETAILS TO STRUCTURAL FRAMING, SIDE LAP CONNECTION DETAILS, LOCATION OF SHORING AND SUPPLEMENTARY SUPPORT STEEL, AS REQUIRED.
- M4 NOT USED
- M5 NOT USED
- M6 NOT USED
- M7 WELD DECKING TO STRUCTURAL STEEL BY CERTIFIED WELDERS USING PREQUALIFIED PROCEDURES. THE ERECTOR SHALL ESTABLISH A WELDING PROCEDURE FOR THE RIDGE WELD OF STEEL DECKING TO THE STRUCTURAL STEEL FOR THE PARTICULAR GAGES USED. PRIOR TO THE START OF DECKING OF THE STEEL DECK, QUALIFY EACH WELDER USING THIS PROCEDURE AS WITNESSED BY THE OWNER'S TESTING LABORATORY.
- M8 FASTEN FLOOR DECKING AT 12 INCHES MAXIMUM ON CENTER TO THE SUPPORTING STEEL WITH A 5/8 INCH DIAMETER WELD. FASTEN SIDE LAPS AT 36 INCHES MAXIMUM ON CENTER OR MID-SPAN, WHICHEVER IS LESS.
- M9 FASTEN ROOF DECKING IN A 3R/5 PATTERN TO THE SUPPORTING STEEL WITH 5/8 INCH DIAMETER WELD. WELD SIDE LAPS AT 36 INCHES MAXIMUM ON CENTER OR MIDSPAN, WHICHEVER IS LESS.
- M10 MECHANICAL FASTENERS MAY BE SUBSTITUTED FOR WELDS IF INFORMATION IS PROVIDED TO THE ARCHITECT/ENGINEER CONFIRMING EQUIVALENT DIAPHRAGM STRENGTH AND STIFFNESS IS ACHIEVED.
- M11 PROVIDE CONTINUOUS SHEET METAL CLOSURES AT ALL SLAB OPENINGS AND SLAB EDGES AND CONTINUOUS DECK CLOSURES AT ALL DECK ENDS. PROVIDE COLUMN CLOSURES, CHAMF STRIPS, RECESSED DRAIN SLUMP PANS, ETC. PROVIDE SUPPLEMENTAL FRAMING AT OPENINGS AS REQUIRED FOR SUPPORT OF METAL DECK. PROVIDE STRAP ANCHORS OR TEMPORARY SHORING AS NECESSARY TO CONTROL CANTILEVER DEFLECTIONS AT FLOOR SLAB EDGES.
- M12 PLACE METAL DECK OVER A MINIMUM OF THREE (3) SPANS IN THE DIRECTION INDICATED; USE SINGLE SPANS ONLY WHERE REQUIRED BY FRAMING GEOMETRY.
- M13 THE ASSIGNED CONSTRUCTION LINE LOAD USED IN DESIGN IS A 20 PSF UNIFORM LOAD OR A 150-POUND CONCENTRATED LOAD ON A 1'-0" WIDE SECTION OF DECK. DO NOT EXCEED THE ASSIGNED CONSTRUCTION DESIGN LINE LOAD WITHOUT FIRST TAKING PROPER SAFETY PRECAUTIONS, INCLUDING TEMPORARY SHORING. FOLLOW ALL APPLICABLE LOCAL AND ASI REQUIREMENTS.

**METAL DECK PROPERTIES**

DECK TYPE	GAGE	I	S <sub>x</sub>	S <sub>y</sub>
1.5" TYPE "B" ROOF DECK	18	0.282	0.318	0.327

- S21 PROVIDE NEW MATERIAL CONFORMING TO THE FOLLOWING REQUIREMENTS FOR ALL STRUCTURAL STEEL:
- MEMBER GRADE
- WIDE FLANGE SHAPES, WTS ASTM A-992
- CHANNELS & ANGLES ASTM A-36
- PIPE TUBULAR SHAPES (ROUND & RECT.) ASTM A-53 GRADE B  
ASTM A-500 GRADE B
- BASE PLATES ASTM A-572 GRADE 42
- ALL OTHER STEEL MEMBERS ASTM A-36
- HIGH STRENGTH BOLTS, NUTS AND WASHERS ASTM A-325 OR A-490 (MIN. 3/4" DIAMETER)
- ANCHOR RODS ASTM F1554 GR 36 OR GR 105
- WELDING ELECTRODE E70XX
- METAL DECK WELDING ELECTRODE E80XX MIN.

**STEEL JOIST NOTES**

- SJ1 NOT USED
- SJ2 A QUALITY CONTROL PROGRAM OF FIELD TESTING AND INSPECTION WILL BE PERFORMED ON ALL STEEL JOIST ERECTION AND CONNECTIONS IN ACCORDANCE WITH THE SPECIFICATIONS. SCHEDULE ALL WORK AND PROVIDE ACCESS TO ALLOW TESTING REQUIREMENTS TO BE COMPLETED. DO NOT ERECT DAMAGED JOISTS.
- SJ3 STEEL JOISTS INCLUDE ALL PROPRIETARY JOISTS (K-SERIES, LH-SERIES AND DLH-SERIES) MANUFACTURED TO CONFORM TO ALL CRITERIA OF THE STEEL JOIST INSTITUTE "STANDARD SPECIFICATIONS", LATEST EDITION. DESIGN STEEL JOISTS FOR THE REQUIRED DEPTHS, SPANS, AND LOADINGS INDICATED ON THE DRAWINGS. DESIGN EXTENDED ENDS FOR SAME UNIFORM LOAD CAPACITY AS THE MAIN SPAN, UNLESS NOTED. POINT LOADS ARE IN ADDITION TO UNIFORM LOAD CAPACITY REQUIRED BY JOIST DESIGNATION.
- SJ4 DESIGN ALL JOISTS AT ROOF LEVELS FOR AN EQUIVALENT NET UPLIFT FORCE EQUAL TO 40 POUNDS PER SQUARE FOOT UNLESS NOTED OTHERWISE ON PLANS.
- SJ5 SUBMIT DETAILED AND CHECKED SHOP DRAWINGS INDICATING ALL INFORMATION NECESSARY FOR FABRICATION AND ERECTION OF ALL JOISTS, ANCHORAGES, BRIDGING AND ACCESSORIES.
- SJ6 PROVIDE CONTINUOUS BRIDGING FASTENED DIRECTLY TO EACH JOIST (HORIZONTAL BRIDGING FOR K-SERIES JOISTS, DIAGONAL BRIDGING FOR LH-SERIES AND DLH-SERIES JOISTS) IN ACCORDANCE WITH STEEL JOIST INSTITUTE RECOMMENDATIONS.
- SJ7 USE DOUBLE ANGLES FOR CHORD MEMBERS. ROUND BOTTOM CHORDS ARE NOT ACCEPTABLE.
- SJ8 THE MINIMUM THICKNESS FOR CHORD MEMBERS, WEB MEMBERS AND BRIDGING IS 7/16 INCH. SPURCE INDIVIDUAL CHORD MEMBERS IN ALTERNATE PANELS. FIELD SPURCES ARE UNACCEPTABLE WITHOUT PRIOR REVIEW AND ACCEPTANCE BY THE ARCHITECT/ENGINEER.
- SJ9 HANG LOADS FROM STEEL JOISTS ONLY AT INTERSECTING POINTS OF WEB AND CHORD MEMBERS. SUSPEND LOADS ONLY WITH ACCEPTABLE JOIST HANGER DEVICES. DO NOT HANG LOADS GREATER THAN THE LOADS NOTED ON THE DRAWINGS.

**MISCELLANEOUS**

- M1 VERIFY EXISTING CONDITIONS AND DIMENSIONS PRIOR TO COMMENCING WORK.
- M2 CONSULT THE ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS FOR LOCATION AND SIZE OF CHASES, INSERTS, OPENINGS, SLEEVES, WASHERS, DRIPS, REVEALS, DEPRESSIONS AND OTHER PROJECT REQUIREMENTS.
- M3 ALL ELEVATIONS ARE REFERENCED TO ELEVATION DATUM.
- M4 COORDINATE ALL FLOOR ELEVATIONS WITH EXISTING FLOOR ELEVATIONS. CONTRACTOR IS TO PROVIDE AND COORDINATE SURVEY OF EXISTING FLOORS PRIOR TO PROCEEDING WITH CONSTRUCTION.

**DESIGN LOADS**

TYPICAL FLOORS LIVE LOAD (INCLUDING PARTITION LOADS)	120 PSF
PUBLIC SPACES, EXIT CORRIDORS, STAIRS AND LOBBIES	100 PSF
COLUMNS	5 PSF
MECHANICAL ITEMS SUSPENDED FROM STRUCTURAL FRAMING	7 PSF

LOADS SUSPENDED FROM COMPOSITE METAL DECK FLOOR SHALL NOT EXCEED 400 LBS. IN ANY 40 SF AREA NOR 10 PSF

**SNOW LOAD (ASCE 7-06)**  
 $P_g = 50.00 \text{ PSF}$   
 $C_e = 1.00$   
 $C_t = 1.00$   
 $I_e = 1.00$   
 $P_f = 38.00 \text{ PSF}$   
 DRIFT LOAD IN ACCORDANCE WITH ASCE 7-06

$P_s = 90 \text{ PSF}$        $P_f = 40.00 \text{ PSF}$

**WIND LOAD (ASCE 7-06)**  
 BASIC WIND SPEED = 90.00 MPH  
 WIND DIRECTIONALITY FACTOR,  $K_d = 0.85$  MAIN WIND FORCE RESISTING SYSTEM (MWRFS)  
 $K_d = 0.85$  COMPONENTS AND CLADDING

BUILDING CLASS = II  
 IMPORTANCE FACTOR,  $I = 1.07$   
 EXPOSURE CATEGORY = B  
 TOPOGRAPHIC FACTOR,  $K_{zt} = 1.00$   
 GUST EFFECT FACTOR,  $G_e = 0.85$   
 INTERNAL PRESSURE COEFFICIENT,  $C_{pi} = +/- 0.18$

**CLADDING WIND PRESSURES**

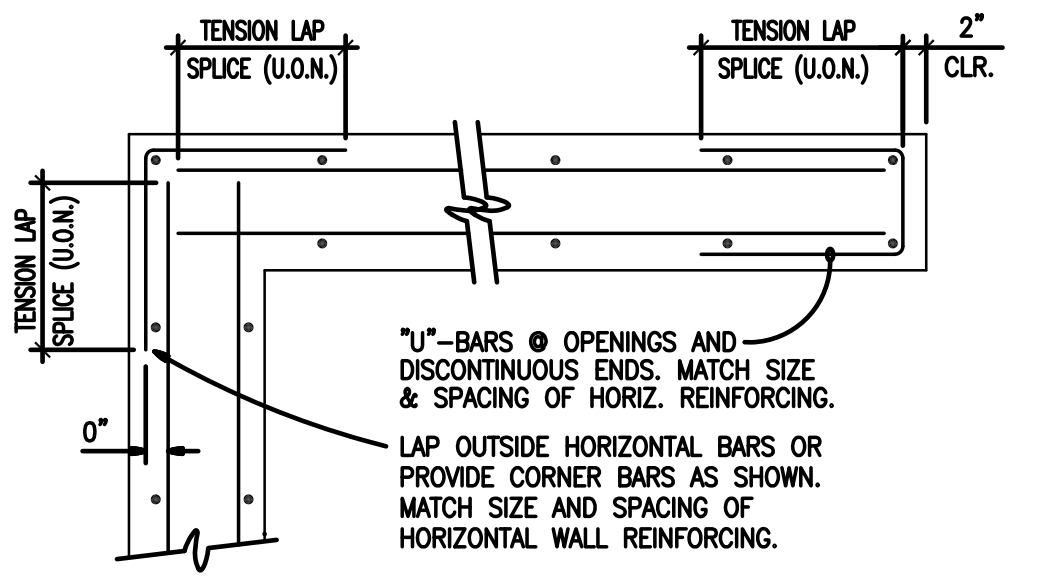
WINDWARD WALL CORNERS =	23.0 PSF
WINDWARD WALL CORNERS =	31.0 PSF
WINDWARD WALL FIELD =	18.0 PSF
LEEWARD WALL FIELD =	25.0 PSF
ROOF CORNER =	58.0 PSF
ROOF FIELD =	38.0 PSF
ROOF OVERHANG =	23.0 PSF
PARAPET =	81.0 PSF

**SEISMIC LOADS (ASCE 7-06)**  
 SITE CLASS "C"  
 OCCUPANCY CATEGORY = II  
 SEISMIC USE GROUP = I  
 OCCUPANCY IMPORTANCE FACTOR = 1.00  
 $S_s = 0.30$   
 $S_d = 2.34$   
 $S_{ds} = 0.312$   
 $S_1 = 0.07$   
 $F_v = 3.50$   
 $S_{d1} = 0.098$   
 $R$  (NORTH SOUTH) = 2.00 - DETAILED MASONRY SHEAR WALLS  
 $R$  (EAST WEST) = 2.00 - DETAILED MASONRY SHEAR WALLS  
 SIMPLIFIED SEISMIC BASE SHEAR  
 BASE SHEAR= 15.6X W  
 SEISMIC DESIGN CATEGORY "C"

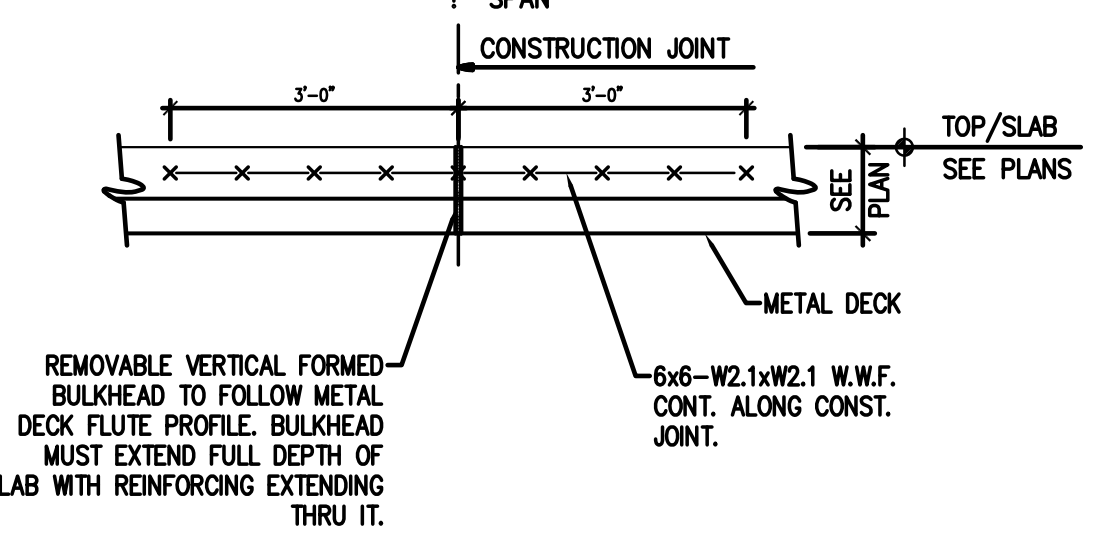
**SOIL LOAD**  
 EQUIVALENT LATERAL SOIL PRESSURE 65 PCF

**ABBREVIATIONS**

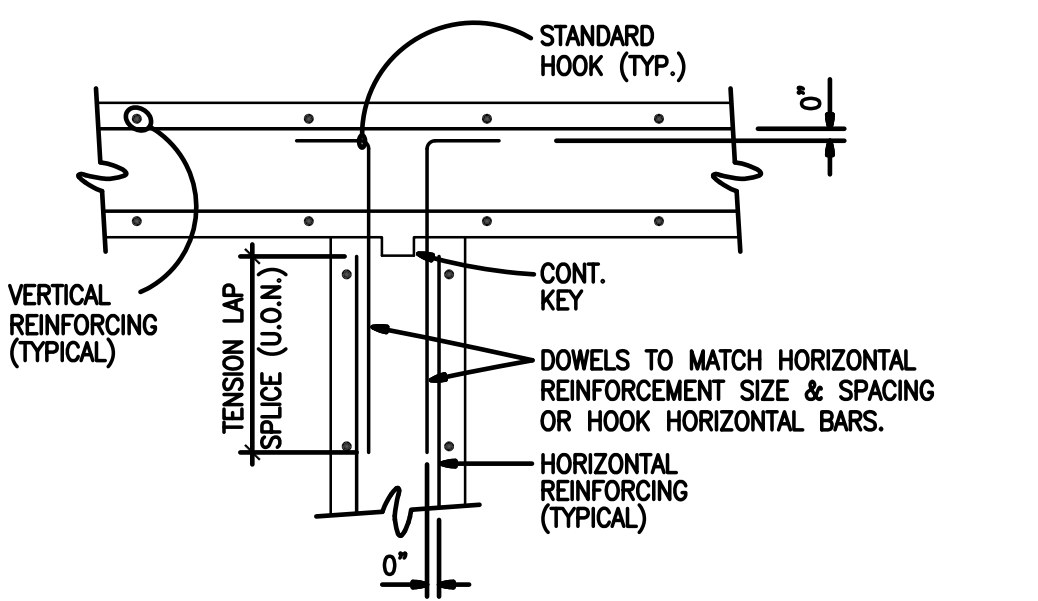
ADJ. ADJACENT	F.D. FLOOR DRAIN	RE. REFER TO
ADVL. ADDITIONAL	FTG. FOOTING	REIN.F. REINFORCE (R) (ING)
ALT. ALTERNATE	FOUND. FOUNDATION	REQ'D. REQUIRED
ALUM. ALUMINUM	GALV. GALVANIZED	REV. REVISION
A.B. ANCHOR BOLT	GAGE. GAGE	R.D. ROOF DRAIN
APPROX. APPROXIMATE	HORIZ. HORIZONTAL	RO. ROOF OPENING
ARCHV. ARCHITECTURAL	H.P. HIGH POINT	SECT. SECTION
BSMT. BASEMENT	I.D. INSIDE DIAMETER	SM. SIMILAR
BM. BEAM	INSUL. INSULATE (D) (ON)	SPEC. SPECIFICATIONS
BLK. BLOCK	LG. LONG	S.O.G. SLAB-ON-GRADE
BOT. BOTTOM	LTL. LINTEL	STD. STANDARD
BRK. BRK. CENTER LINE	LLH. LONG LEG HORIZONTAL	STL. STEEL
CL. CENTER LINE	LLV. LONG LEG VERTICAL	STRUCT. STRUCTURAL
CP. CAST-IN-PLACE	LP. LOW POINT	S.W. SHORT WAY
CEM. CEMENT	L.W. LONG WAY	T&B. TOP & BOTTOM
CLR. CLEAR	L.V. LOW VALLEY	T/. THICK (NESS)
C.O. CLEAN OUT	M.O. MASONRY OPENING	TOS. TOP OF STEEL
C.S. COLUMN STRIP	M.S. MIDDLE STRIP	T.O.W. TOP OF WALL
COL. COLUMN	MATL. MATERIAL	TYP. TYPICAL
CONC. CONCRETE	MAX. MAXIMUM	T.O.L. TOP OF LEVEL UNLESS OTHERWISE NOTED
CONJ. CONCRETE MASONRY UNIT	MECH. MECHANICAL	V.F. VERIFY IN FIELD
CONT. CONTINUOUS	MIN. MINIMUM	VERT. VERTICAL
C.J. CONTROL JOINT OR CONSTRUCTION JOINT	MISC. MISCELLANEOUS	W.P. WORKING POINT
DET. DETAIL	N.T.S. NOT TO SCALE	W/ WITH
DIAG. DIAGONAL	NOM. NOMINAL	W/O WITHOUT
DIA. DIAMETER	NUM. NUMBER	W.D. WOOD
DIM. DIMENSION	O.C. ON CENTER	W.D. DIA. WOOD DIA.
DWG. DRAWING	OPNG. OPENING	
E.A. EACH	O.D. OUTSIDE DIAMETER	
ELEV. ELEVATION	PL. PLATE	
EQU. EQUAL	POF. POUNDS PER CUBIC FOOT	
E.O.D. EDGE OF DECK	PSF. POUNDS PER SQUARE INCH	
E.O.S. EDGE OF SLAB	PPF. POUNDS PER SQUARE FOOT	
EXP. EXPANSION	PCP. PRECAST CONCRETE PANEL	
EXST. EXISTING	PSC. PRE-STRESS CONCRETE	
E.A. EXPANSION JOINT	QTY. QUANTITY	
FB. FACE BRICK	R. RADIUS	



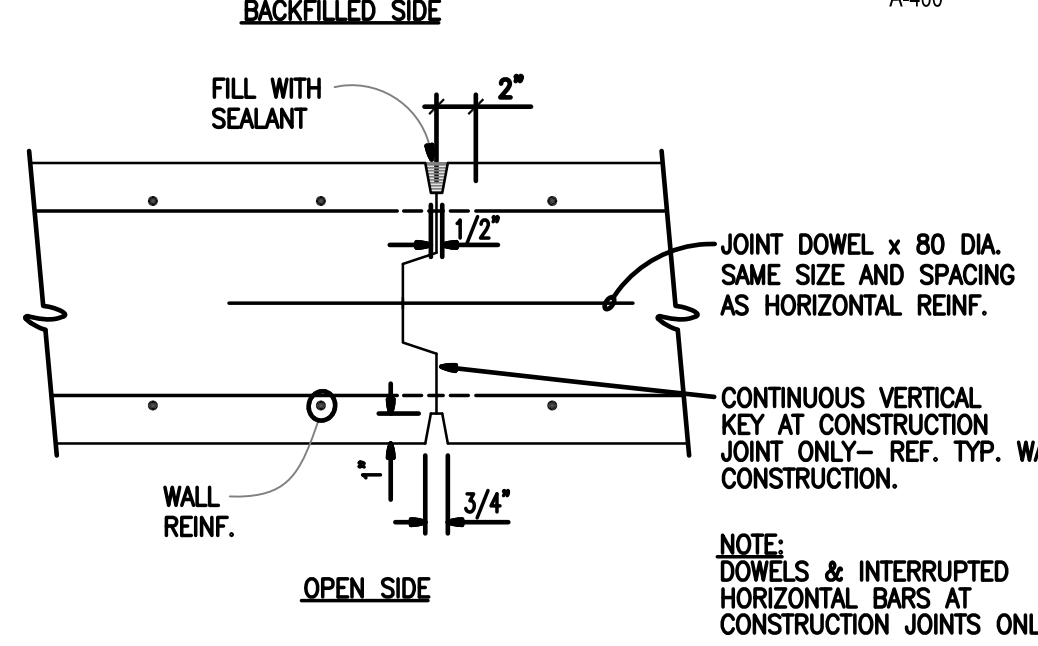
**4 TYPICAL CORNERS AND ENDS OF CONCRETE WALLS**  
NO SCALE



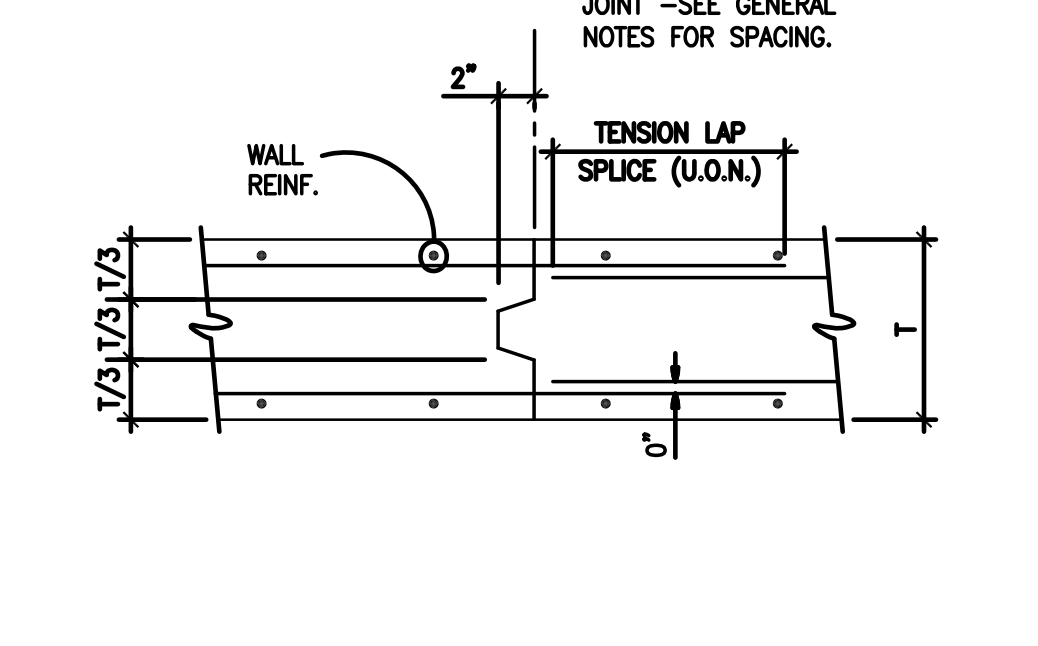
**7 TYPICAL BLENDED FIBER REINFORCED COMPOSITE SLAB CONSTRUCTION JOINT**  
NO SCALE



**3 TYPICAL CONCRETE WALL INTERSECTION**  
NO SCALE



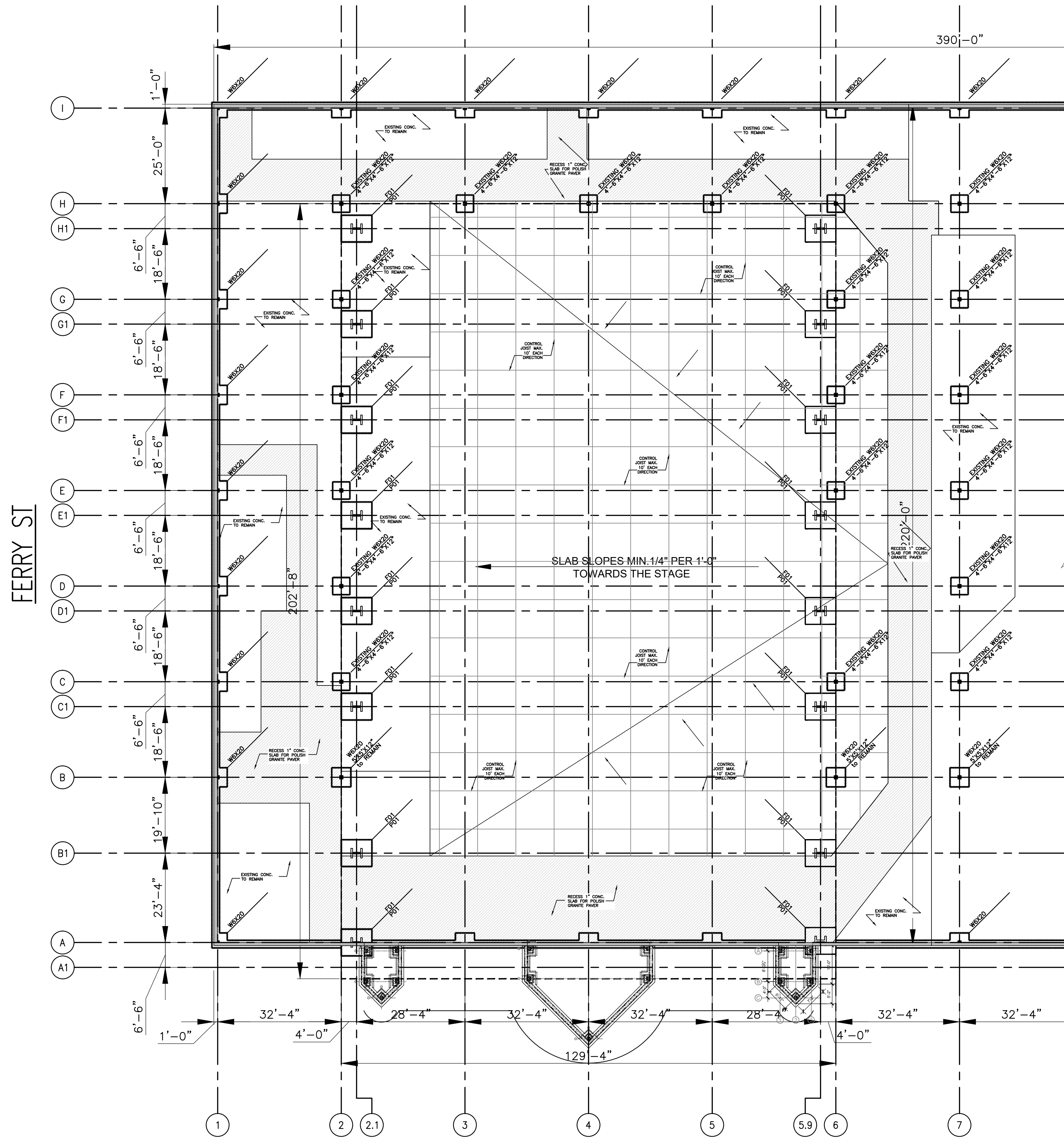
**2 TYPICAL WALL CONTROL JOINT**  
NO SCALE



**1 TYPICAL WALL CONSTRUCTION JOINT**  
NO SCALE

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<p>JOB NO.: 20-2400      DRAWN BY: HF</p>		<p><b>GENERAL NOTES &amp; STRUCTURAL DETAILS</b></p>			
NO.	DATE	DESCRIPTION	NO.	DATE	DESCRIPTION
<p>SCALE:      DATE: 11/11/2020      DWG. S-000</p>					

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FOOTING SCHEDULE					
MARK	WIDTH	LENGTH	THICKNESS	REINFORCING (T&B), EACH WAY U.O.N.	REMARKS
F01	8'-0"	7'-0"	2'-0"	8-#6 LONG 8-#6 SHORT	

PIER SCHEDULE				
MARK	SIZE	VERT REINF	TIES	REMARKS
P01	36x30	(14)-#6	#3@4"	

ANCHOR ROD SCHEDULE			
MARK	THICK	SIZE	ANCHOR RODS
BP01	1.25"	22x22	(6) 1.50"Øx 24" EMBED GR36
BP02	1.25"	22x12	(4) 1.00"Øx24" EMBED GR36

**PROPOSED FOUNDATION PLAN** 1  
 SCALE: 1/16"=1'-0" S-2

LEGEND	
-----	PROPOSED STRUCTURAL BOUNDARY

NO.	DATE	DESCRIPTION	NO.	DATE	DESCRIPTION
1.	10/22/21	REVISION			

IF IS A VIOLATION OF SECTION 2209, SUBDIVISION 2, OF THE NEW YORK STATE EDUCATION LAW FOR ANY PERSON OTHER THAN THOSE WHOSE SIGN APPEARS ON THIS DRAWING, TO ALTER IN ANY MANNER ANY ITEM ON THIS DRAWING. IF AN ITEM IS ALTERED, THE ALTERING ENGINEER SHALL AFFIX TO THE ITEM HIS SIGN AND THE NOTATION "ALTERED BY" FOLLOWED BY HIS SIGNATURE AND THE DATE OF SUCH ALTERATION, AND A BRIEF DESCRIPTION OF THE ALTERATION.

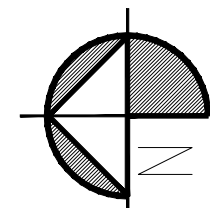
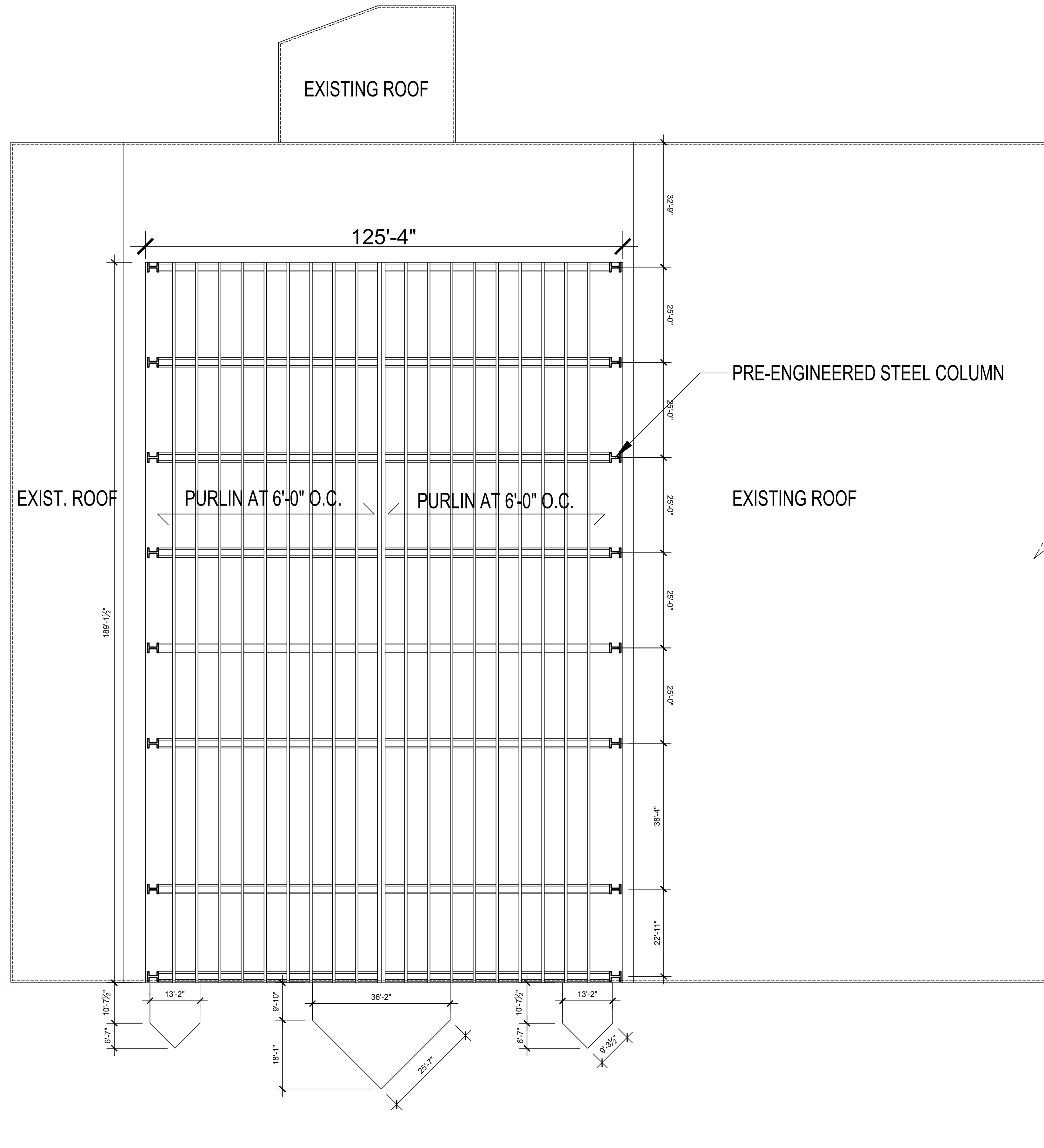


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JOB NUMBER # 20-2400  
**PROPOSED FOUNDATION PLAN**  
 SCALE: AS NOTED    DATE: 03-20-2021    DWG. **S-101**

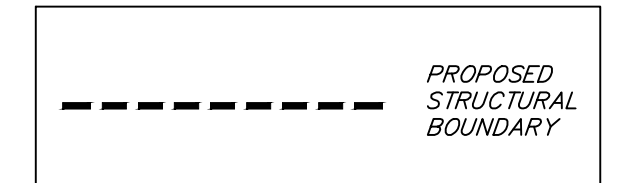


**STRUCTURAL PLAN**

SCALE: 1/16" = 1'-0"

1  
S-1

**LEGEND**



NO.	DATE	DESCRIPTION	NO.	DATE	DESCRIPTION
11	10/22/21	GEN REVISION			

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JOB NUMBER # 20-2400  
**EXISTING FOUNDATION PLAN**  
 SCALE: AS NOTED    DATE: 03-20-2021    DWG. **S-101**